# Short Communication

Seepage under a Dam on Sloping Soil with Discontinuities in Slopes

by

A. Siva Reddy\*

K. Sridharan\*\*

V. G. Kulkarni\*\*\*

#### Introduction

Herein, the influence on the flow characteristics of the discontinuities in the slopes overlying and underlying the porous medium of finite depth has been studied. Such geometry may occur in several field situations. One example is a major project undertaken by the Government of Karnataka, namely, the Chakra dam. It is being constructed across the Chakra river situated in the valley adjoining the Linganamakki catchment area to divert the river water to the Sharavathi valley to augment power supply for the Kudremukh Iron Ore Project.

## Analysis

Fig 1 (a) shows the flow domain in the physical plane, for which Z = x + iy. The region ABCDEFGA is transformed onto the lower half of the auxiliary t plane, (Fig 1 (c)), using Schwarz-Christoffel transformation. The relationship for the mapping is obtained as follows

$$Z = M \int \frac{dt}{t^{1-p} (t-a)^{1-\sigma} (t-\gamma) (t-\delta)^{\sigma-1} (t-1)^{p-1}} + N \qquad \dots (1)$$

in which M and N are arbitrary constants, the vertices of the polygon ABCDEFGA in the Z plane being mapped onto  $-\infty$ , 0,  $\alpha$ ,  $\beta$ ,  $\gamma$ ,  $\delta$ , 1,  $\infty$  respectively, in the t plane.

Integrating Equation (1) for  $0 < t < \alpha$  [Gradshteyn and Ryzhik, 1965] and noting that  $Z = Z_c$  at  $t = \alpha$ ,  $B_1$  (See Fig 1) can be obtained as follows.

$$B_1 = |Z_c| = |M| |(-1)^{\rho} \alpha^{\rho+\sigma-1} \gamma^{-1} \delta^{1-\sigma} \sum_{n=0}^{n=\infty} A_n B(n+\rho,\sigma).$$

$$F_1(n+\rho, \rho-1, 1; n+\rho+\sigma; a, \frac{a}{\gamma})$$
 | ...(2)

Civil Engineering Department, Indian

\*\* Assistant Professor

\*\*\* Formerly Research Scholar J Institute of Science, Banglore, 560012, India (This short Communication was received in October 1983, and is open for discussion till the end of June 1984)

Professor





(a) z plane(b) w plane

(c) t plane.

in which

$$A_n = \frac{(\sigma-1)(\sigma)....(\sigma-1+n-1)}{n} \left(\frac{a}{\delta}\right)^n; n = 1, 2...$$

and  $A_o = 1$ .

B ( ) = Complete beta function

 $F_1()$  = Hypergeometric function of two variables.

Integrating Eq. (1) for  $a < t < \gamma$  and noting that  $Z = Z_D$  at  $t = \beta$ ,  $B_2$  (See Fig. 1) can be obtained as

$$B_{2} = |Z_{D}-Z_{\sigma}| = |(-1)^{1-\rho-\sigma} M_{\gamma}^{\rho-1} (\gamma-a)^{\sigma-1} (\delta-a)^{1-\sigma} (1-a)^{1-\rho}$$

$$\sum_{n=0}^{n=\infty} B_{n} (\gamma-a)^{n} \sum_{p=0}^{n=\infty} C_{p} I_{p} | ...(3)$$

in which

$$B_{n} = \frac{(1-\rho)(2-\rho)\dots(n-\rho)}{n!} \frac{1}{\gamma^{n}} n = 1, 2...$$

$$B_{o} = 1;$$

$$C_{p} = C'_{p} C''_{0} + C'_{p-1} C''_{1} + \dots + C'_{1} C''_{p-1} + C'_{0} C''_{p}$$

$$p = 0, 1, 2, \dots,$$

$$C'_{p} = \frac{(\sigma-1)(\sigma)(\sigma+1)\dots(\sigma-1-p-1)}{p} \left(\frac{\gamma-\alpha}{\delta-\alpha}\right)^{p}$$

$$p = 1, 2, \dots,$$

$$C''_{p} = \frac{(-1)^{p}(1-\rho)(-\rho)\dots(1-\rho-p-1)}{p} \left(\frac{\gamma-\alpha}{1-\alpha}\right)^{p}$$

$$p = 1, 2, \dots,$$

$$C''_{0} = 1;$$

$$C''_{0} = 1;$$

$$C''_{0} = 1;$$

$$I_{p} = \sum_{r=0}^{r=\infty} \frac{\left(\frac{t-\alpha}{\gamma-\alpha}\right)^{p+\sigma+r}}{p+\sigma+r}; r = 0, 1, 2, \dots,$$
for  $n=0$ 

$$= B_{r} (p+\sigma, n) \text{ for } n > 0;$$

$$B_{r} ( ) = \text{ incomplete beta function.}$$

Again, integrating Eq. (1) for  $\delta < t < 1$  and introducing the condition  $Z = Z_F$  at  $t = \delta$ ,  $B_{\mathfrak{g}}$  (See Fig. 1) is obtained as

INDIAN GEOTECHNICAL JOURNAL

$$B_{3} = |Z_{F}-Z_{G}| = |(-1)-\rho M (1-\alpha)\sigma-1 (1-\gamma)-1 (1-\delta)^{3-\rho-\sigma}$$

$$\sum_{n=0}^{n=\infty} D_{n} B (n+2-\rho, 2-\sigma).$$

$$F_{1}(n+2-\rho, 1-\sigma, 1; n+4-\rho-\sigma; \frac{1-\delta}{1-\alpha}; \frac{1-\delta}{1-\gamma}) | \dots (4)$$

in which

$$D_n = \frac{(1-\rho) (2-\rho) \dots (n-\rho)}{n!} (1-\delta)^n :$$
  
 $n = 1, 2, \dots$ 

 $D_o = 1$ .

As t passes around a semicircle of small radius at point E in the t plane the corresponding change in Z is given by

$$-T_2 \cos \theta_2 \left[ \sin \theta_2 + i \cos \theta_2 \right]$$

From this condition,

$$T_{2} = \frac{-i (-1)^{1-\rho-\sigma} \pi M_{\gamma}\rho - 1 (\gamma-\alpha)\sigma - 1 (\delta-\gamma)^{1-\sigma} (1-\gamma)^{1-\rho}}{(\cos \theta_{2} \sin \theta_{2} + i \cos^{2} \theta_{2})} \dots (5)$$

From the condition at point A,

$$T_1 = -M\pi \qquad \dots (6)$$

For the case  $T_1 = T_2 = T$ , eliminating T from Eqs. (5) and (6).

$$\delta = \gamma + (\gamma - \alpha) \left(\frac{\gamma}{1 - \gamma}\right)^{\frac{1 - \rho}{1 - \sigma}} \left[\cos^2 \theta_2 \cos (\sigma + \rho) \pi - \cos \theta_2 \sin \theta_2 \sin (\rho + \sigma) \pi\right]^{\frac{1}{1 - \sigma}} \dots (7)$$

For this case with points G and F being vertically below point B and C, respectively,  $B_1$  and  $B_3$  are equal for the angular relations shown in Fig 1 (a). The unknowns  $\alpha$ ,  $\gamma$ , and  $\delta$  are obtained by using Eqs. (2), (4) and (7) for the given physical quantities  $B_1$ ,  $B_3$  and T.  $\beta$  which corresponds to  $B_2$  is obtained from Eq. (3).

#### Mapping the Complex Potential Plane

In the complex potential plane (w plane) the boundaries of the flow region from a rectangle (Fig. 1(b)). The complex potential, w. is defined as

$$w = \phi + i \Psi$$

in which, the velocity potential,  $\phi = -k \left( \frac{p_i}{\gamma_w} + y \right) + C_1$ 

k = coefficient of permeability,

84

 $p_i$  = pressure at the point being considered with  $\gamma$  as coordinats,

 $\gamma_w =$  unit weight of water and

 $\Psi =$  stream function

 $C_1 = \text{constant.}$ 

The mapping of the w plane onto the lower half of the t plane, according to Schwarz-Christoffel transformation is given by

$$w = M' \int \frac{dt}{t^{1/2} (t-\beta)^{1/2} (t-\gamma)^{1/2}} + N' \qquad \dots (8)$$

in which M' and N' are constants.

Using the conditions w = 0 at  $t = \beta$  and w = iq at  $t = \gamma$  in Eq. (8), the quantity of seepage in non dimensional form is obtained as

$$\frac{q}{kH} = \frac{K\left(\frac{\sqrt{\lambda-\beta}}{\gamma}\right)}{K\left(\frac{\sqrt{\beta}}{\gamma}\right)} \dots (9)$$

in which K() = complete elleptic integral of first kind.

## Exit Gradient

The exit gradient,  $I_E$ , perpendicular to the downstream boundary is given by

$$I_{B} = \frac{1}{k} \left( \frac{dw}{dt} \quad \frac{dt}{dz} \right) (Cos\theta_{3} + iSin\theta_{3}) \qquad \dots (10)$$

in which  $\theta_3$  is the angle made by a streamline with the x axis at exit point.  $\frac{dz}{dt}$  and  $\frac{dw}{dt}$  are obtained from Eq. (1) and (8). Hence, Eq. (11) becomes

$$|\frac{I_ET}{H}| = |\frac{\pi\sqrt{\gamma}}{2k\left(\sqrt{\frac{\beta}{\gamma}}\right)}\frac{t^{\frac{1}{2}-\rho}(t-a)^{1-\sigma}(\gamma-t)^{\frac{1}{2}}(\delta-t)^{\sigma-1}(1-t)^{\rho-1}}{(t-\beta)^{\frac{1}{2}}}$$
...(11)

To check the results obtained, finite difference technique is used and the resulting simultaneous equations are solved by Successive Over Relaxation method.

The results of the analytical method were also checked for a particular case by electrical analog method.

## **Results and Discussions**

In the present analysis, points G and F in the physical plane (z plane) are assumed to be directly below points B and C, respectively;  $T_1$  and  $T_2$ 







![](_page_6_Figure_2.jpeg)

87

are assumed to be equal. Knowing the physical parameters H, k,  $B_2$ ,  $B_1$ T,  $\rho$  and  $\sigma$ , and the transformation relations, the information regarding quantity of seepage and distribution of exit gradient can be obtained. The variables,  $\alpha$ ,  $\beta$  and  $\gamma$ , in t plane were obtained by solving Eq. (2), (3) uations and (4).

Numerical results are presented for quantity of seepage and exit gradient distribution in nondimensional form for the ranges of  $\theta_1$  from 5° to 20° and  $\theta_2$  from 0° to 20°.

Figs. 2 and 3 illustrate the relation between q/kH and  $B_2/T$ . It is observed from the curves that at constant values of  $\theta_1$  and  $\theta_2$ , the reduction id q/kH with increase in  $B_2/T$  is relatively more at smaller values of  $B_1/T$ than when  $B_1/T$  is very large.

Comparison of curves for q/kH versus  $B_2/T$  for different sets of  $\theta_1$  and  $\theta_2$  indicates that the discontinuities in slopes do not have much influence on the seepage quantity. However, an increase in  $\theta_1$ , for given values of  $\theta_2$ ,  $B_1/T$  and  $B_2/T$ , causes a small decrease in q/kH, whereas, an increase in  $\theta_2$  for given values of  $\theta_1$ ,  $B_1/T$  and  $B_2/T$  causes a small increase in q/kH. For instance, for given  $\theta_2 = 10^\circ$ ,  $B_1/T = 0.5$  and  $B_2/T = 0.5$  the corresponding values of q/kH for  $\theta_1 = 5^\circ$  and  $20^\circ$  are 0.5484 and 0.5317, i.e., the percentage decrease in q/kH is 3.05; while for given  $\theta_1 = 10^\circ$  and for the same values of  $B_1/T$  and  $B_2/T$ , q/kH increases from 0.5244 to 0.5518 when  $\theta_2$  changes from 5° to 20°, i.e., the percentage increase in q/kH is 5.23.

![](_page_7_Figure_5.jpeg)

FIGURE 4 Comparison of Results of Analytical, Experimental and Numerical Methods for Distribution of Exit Gradient

When  $\theta_1 = \theta_2 = 0^\circ$  and for  $B_1/T = 0.5$  and  $B_2/T = 0.5$ , the obtained value of q/kH = 0.53375 compares well with Pavlovsky's solution (q/kH = 0.53307) for same conditions (vide Harr, 1962).

Fig. 4 shows the comparison of results for exit gradient distribution obtained under similar conditions, from analytical method, numerical method and electrical analogy method. It is seen from Fig. 8, that the results of analtical method check satisfactorily with the results of numerical method and those of electrical analogy method.

# Conclusions

An analytical solution has been presented for the problem of seepage under a dam located on sloping soil with discontinuities in slopes. Numerical results are presented for quantity of seepage and the exit gradient for  $\theta_1$  and  $\theta_2$  values between 5° and 20°. For this range, it is observed that an increase in  $\theta_1$  causes a decrease in q/kH while an increase in  $\theta_2$  causes an inerease in q/kH; but, however, the increase or decrease in q/kH is found to be small. Thus, the discontinuities in slopes do not considerably influence the quantity of seepage.

Further, the results obtained from numerical method and electrical analog method compare well with the results of analytical method.

## References

GRADSHTEYN, I.S. and RYZHIK, I.M., (1965), "Table of Integrals, Series and Products". Academic Press, New York.

HARR, M.E., (1962), "Groundwater and Seepage". McGraw-Hiil Book Company Inc, New York.